INPUTS and CONSTANTS

Red and Bonita Mine Bulkhead: 2/10/2015

Tunnel Height (h_t)	7	ft	
Tunnel Width (w_t)	5	ft	
Bulkhead Height (h_b)	10	ft	
Bulkhead Width (w_b)	7	ft	
Design water head (H)	1253	ft	
Bulkhead Trial Thickness (L_T)	15	ft	
Water density(γ_w)	62.4	pcf	
Overburden rock density (γ_r)	165	pcf	
Concrete Density (γ_c)	151	pcf	
Concrete Compressive Strength (f_c)	3,000	psi	
Acceptable bulkhead pressure gradient (p_{ag})	41	psi/ft	
Bulkhead depth below surface (B_w)	203	ft	
Slope Angle of Topography (β)	37		
Accoustical velocity of water (c')	4,748	ft/s @50°F	
Peak Ground Acceleration (PGA)	0.185	g	
Gravity Acceleration (g)	32.2	ft/sec^2	
Seed & Idriss Constant (SI)	1.8044	(ft/sec)/g	From Seed and Idriss
Seismic Design Handbook Constant (SDH)	2	(ft/sec)/g	From Seismic Design
Beam Unit Width (b)	1	ft	
Inby Line-of-Site Water Distance (S_{ls})	125	ft	
Rebar Yield Strength (f_y)	60,000	psi	
Minimum Rebar Cover (m_c)	3.5	in	
Rock Cover Factor of Safety (F_{RC})	1.1	Range 1.1-1.3 (Based on Bergh-C	
Fluid Static Load Factor (ϕ_{fs})	1.4		
Concrete Flexural Strength Reduction Factor (ϕ_{pc})	0.55		
Earthquake Static Fluid Load Acceleration Factor (ϕ_{fe})	1.05		
Earthquake Impounded Fluid Load Acceleration Factor (ϕ_{ea})	1.40		
Reinforced Concrete Flexural Strength Reduction Factor (ϕ_{rc})	0.90		
Rebar Flexural Strenth Reduction Factor (ϕ_{rt})	0.90		

1983
Handbook (pg55)

hriestensen and Dannevig, 1971)

Water Hammer

Approach based on "Permanent Sealing of Tunnels to Retain Tail

Inputs: *Change values on Input Tab*

Accoustical velocity of water (c')	4,748	ft/s @50°F	
Peak Ground Acceleration (PGA)	0.185	g	
Water Density (γ_w)	62.4	pcf	
Gravity Acceleration (g)	32.2	ft/sec^2	
Earthquake Static Fluid Load Acceleration Factor (ϕ_{fe})	1.05		
Seed & Idriss Constant (SI)	1.8044	(ft/sec)/g	From Seed and Idriss 1983
Seismic Design Handbook Constant (SDH)	2	(ft/sec)/g	From Seismic Design Handbo

Calculation:

Max Earthquake Acceleration (α)	α = PGA*g =	5.957	ft/sec^2	
Max Velocity SI (v_{max})	v_{max} = SI*PGA =	0.33381	ft/s	Seed and Idriss
Max Velocity SI (v_{max})	v_{max} = SDH*PGA =	0.37	ft/s	Seismic Design Ha
Water Hammer Pressure (P_H)	$P_H = c'*v_{max}*\gamma_w$ =	109,622	lb	Used SDH
Factored Water Hammer Pressure (P'_H)	$P'_H = P_H*\phi_{fe}$ =	115,103	lb	

ings or Acid Rock Drainage", Lang, 1999.

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Hydrofrac

Inputs:

Design water head (H)	1253	ft
Water density(γ_w)	62.4	pcf
Overburden rock density (γ_r)	165	pcf
Acceptable bulkhead pressure gradient (p_{ag})	41	psi/ft
Bulkhead depth below surface (B_w)	203	ft
Rock Cover Factor of Safety (F_{RC})	1.1	
Slope Angle of Topography (β)	37	degrees

Calculations:

Maximum Hydraulic Pressure Head (p)	$p = H\gamma_w/144 =$	543.0	psi
Minimum Rock Cover Required: Hydrofrac (Z) (Abel Method)	$Z = 144p/2\gamma_r =$	236.9	ft
Minimum Rock Cover Required: Hydrojack (Z) (Norwegian Tunnel Method)	$Z = H\gamma_w F/\gamma_r \cos\beta =$	652.7	ft
Minimum contact grout pressure (σ_{mingp})	$\sigma_{mingp} = B_w\ \gamma_w/144 =$	88.0	psi
Maximum contact grout pressure (σ_{mingp})	$\sigma_{maxgp} = 2B_w\ \gamma_r/144 =$	465.2	psi
Maximum contact grout pressure (σ_{mingp})	$\sigma_{maxgp} = 2B_w\ \gamma_r \cos\beta/144F_{RC} =$	168.9	psi
Required bulkhead thickness for pressure gradient (L_{hp})	$L_{hp} = p/p_{ag} =$	13.2	ft

Punching Shear Design

Inputs:	*Change values on Input Tab*		
Concrete Compressive Strength (f_c)		3,000	psi
Bulkhead Height (h_b)		10	ft
Bulkhead Width (w_b)		7	ft
Design Head (H)		1253	ft
Water Density (γ_w)		62.4	pcf
Fluid Static Load Factor (ϕ_{fs})		1.4	
Factored Water Hammer Pressure (P'_H)		115,103	lb (Calculated from Water Hammer Tab)

Calculations:			
Concrete Shear Strength (f_{cs})	$f_{cs} = 2*f_c^{1/2}=$	109.5	psi
Static Fluid Load on Bulkhead Face (F_s)	$F_s = H*\gamma_w*h_b*w_b=$	5,473,104	lb
Factored Static Fluid Load on Bulkhead (F_s')	$F_s' = F_s*\phi_{fs} =$	7,662,346	lb
Length of Bulkhead Required for Shear (L_s)	$L_s = F_s'/(2*(h_b+w_b)*f_{cs}*144)$	14.29	ft

Earthquake Consideration (Water Hammer):			
Length of Bulkhead Required (L_s)	$L_s = (F_s'+P'_{H})/(2*(h_b+w_b)*f_{cs}*144)$	14.50	ft

Plain Concrete Deep Beam Bending Stress

Inputs:

	Change values on Input Tab			*Change values on Input Tab*	
Concrete Compressive Strength (f_c)	3,000 psi		Peak Ground Acceleration (PGA)	0.185 g	
Bulkhead Height (h_b)	10 ft		Fluid Static Load Factor (ϕ_{fs})	1.4	
Bulkhead Width (w_b)	7 ft		Concrete Flexural Strength Reduction Factor (ϕ_{pc})	0.55	
Tunnel Height (h_t)	7 ft		Earthquake Static Fluid Load Acceleration Factor (ϕ_{fe})	1.05	
Tunnel Width (w_t)	5 ft		Earthquake Impounded Fluid Load Acceleration Factor (ϕ_{ea})	1.40	
Design Head (H)	1253 ft		Beam Unit Width (b)	1 ft	
Inby Line-of-Site Water Distance (S_{ls})	125 ft		Static Fluid Load on Bulkhead Face (F_s)	5,473,104 lb	(Calculated from Punching Shea
Water Density (γ_w)	62.4 pcf		Factored Static Fluid Load On Bulkhead Face (F_s')	7,662,346 lb	(Calculated from Punching Shea
Concrete Density (γ_c)	151 pcf		Factored Water Hammer Pressure (P'_H)	115,103 lb	(Calculated from Water Hamme
Bulkhead Trial Thickness (L_T)	15 ft				

Calculations:			
Deep Beam Verification	w_b/L_t =	0.5	Deep Beam
Uniform Static Fluid Load on Face (f'_s)	$f'_s = F'_s/(h_b*w_b)$ =	109,462 psf	
Maximum Nominal Bending Moment (M_n)	$M_n = f'_s*w_b^2/8$ =	670,455 ft-lb	(f'_s load per unit length w/ 1ft beam width)
Factored Nominal Bending Moment (M'_u)	$M'_u = M_n/\phi_{pc}$ =	1,219,010 ft-lb	
Concrete Flexural (tensile) Design Stress (f_{cl})	$f_{cl} = 3*f_c^{1/2}$ =	164.3 psi	
Plain Concrete Beam Bulkhead Length (L_{st})	$L_{st} = (6*M'_u/b*f_{cl})^{1/2}$ =	17.6 ft	
Considering Earthquake (Water Hammer):			
Factored Earthquake Load on Face (U'_a)	$U'_a = F'_s+P'_H$ =	7,777,449 lb	
Uniform Static Fluid Load on Face (u'_s)	$u'_s = U'_s/(h_b*w_b)$ =	111,106 psf	
Maximum Nominal Bending Moment (M_n)	$M_n = u'_s*w_b^2/8$ =	680,527 ft-lb	(u'_s load per unit length w/ 1ft beam width)
Factored Nominal Bending Moment (M'_u)	$M'_u = M_n/\phi_{pc}$ =	1,237,321 ft-lb	
Concrete Flexural (tensile) Design Stress (f_{cl})	$f_{cl} = 3*f_c^{1/2}$ =	164.3 psi	
Plain Concrete Beam Bulkhead Length (L_{st})	$L_{st} = (6*M_u/b*f_{cl})^{1/2}$ =	17.7 ft	
Considering Earthquake (Abel Method):			
Factored Earthquake Accelerated Static Fluid Load (E_{fe})	$E_{fe} = F_s*\phi_{fe}$ =	5,746,759 lb	
Factored Earthquake Accelerated Line-of-Sight Fluid Load (E_{fm})	$E_{fm} = S_{ls}*\gamma_w*h_t*w_t*PGA*\phi_{ea}$ =	70,707 lb	
Factored Earthquake Bulkhead Load (E_{bm})	$E_{bm} = L_{st}*\gamma_c*h_b*w_b*PGA*\phi_{ea}$ =	41,064.45 lb	
Factored Earthquake Load on Face (U'_a)	$U'_a = E_{fe}+E_{fm}+E_{bm}$ =	5,858,531 lb	
Uniform Static Fluid Load on Face (u'_s)	$u'_s = U'_s/(h_b*w_b)$ =	83,693 psf	
Maximum Nominal Bending Moment (M_n)	$M_n = u'_s*w_b^2/8$ =	512,621 ft-lb	(u'_s load per unit length w/ 1ft beam width)
Factored Nominal Bending Moment (M'_u)	$M'_u = M_n/\phi_{pc}$ =	932,039 ft-lb	
Concrete Flexural (tensile) Design Stress (f_{cl})	$f_{cl} = 3*f_c^{1/2}$ =	164.3 psi	
Plain Concrete Beam Bulkhead Length (L_{st})	$L_{st} = (6*M_u/b*f_{cl})^{1/2}$ =	15.4 ft	

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Reinforced Concrete Deep Beam Bending Design:

Inputs: *Change values on Input Tab*

Reinforced Concrete Flexural Strength Reduction Factor (ϕ_{rc})	0.90	
Rebar Flexural Strenth Reduction Factor (ϕ_{rt})	0.90	
Concrete Compressive Strength (f_c)	3,000	psi
Beam Unit Width (b)	1	ft
Rebar Yield Strength (f_y)	60,000	psi
Maximum Nominal Bending Moment (M_n)	680,527	ft-lb (Plain Concrete Deep Be
Bulkhead Trial Thickness (L_T)	15	ft
Minimum Rebar Cover (m_c)	3.5	in

Calculations:

Compressive Force (C)	$C = \phi_{rc}*f_c*b*a =$	32,400	a (psi)
Tensile Force (T)	$T = A_s*f_y =$	60,000	A_s (psi)
Minimum Concrete Depth to Balance Rebar (a)	a =	1.852	A_s (psi)
Factored Bending Moment (M'$_u$)	$M'_u = M_n/\phi_{rt} =$	756,141	ft-lb
Factored Bending Moment (M'$_u$)	$M'_u = M_n/\phi_{rt} =$	9,073,690	in-lb
Maximum Rebar Cover (d)	$d = 12*L_T-m_c =$	176.5	in
$C_1A_s^2-C_2A_sd+M'_u = 0$			
	$C_1 = f_y*a/2 =$	55,556	
	$C_2 = f_y*d =$	-10,590,000	
	$C_3 = M'_u =$	9,073,690	
Area of Steel Required (A_s)	$A_s = (-C_2-(C_2^2-4*C_1*C_3)^{1/2})/2*C_1 =$	0.861	in^2/ft

Bar Size (#)	9	(enter value)
Spacing (C-C)	9	in (enter value)
Area of Steel (A_s)	1.33	in^2/ft

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